



JOURNAL OF WATER AND LAND DEVELOPMENT

e-ISSN 2083-4535



Polish Academy of Sciences (PAN)

Institute of Technology and Life Sciences - National Research Institute (ITP - PIB)

JOURNAL OF WATER AND LAND DEVELOPMENT DOI: 10.24425/jwld.2022.143723 2022, Special Issue: 80–90

Assessment of the reliability of the operation of a sewage treatment plant using Monte Carlo simulation

Karolina Migdał¹⁾ ⊠ (1), Agnieszka Operacz¹⁾ (1), Iryna Vaskina²⁾ (1), Paulina Śliz³⁾ (1), Jorge Tavares^{4,5)} (1), Adelaide Almeida^{4,5,6)} (1), Michał Migdał⁷⁾

¹⁾ University of Agriculture in Krakow, Faculty of Environmental Engineering and Land Surveying, Department of Sanitary Engineering and Water Management, al. Mickiewicza 24/28, 30-059 Kraków, Poland

²⁾ Sumy State University, Faculty of Technical System and Energy Efficient Technologies, Department of Applied Ecology, Sumy, Ukraine

³⁾ Cracow University of Economics, Institute of Spatial Management and Urban Studies,
Department of Spatial Management, Kraków, Poland

⁴⁾ Polytechnic Institute of Beja, Department of Technology and Applied Sciences, Beja, Portugal

5) University of Beira Interior, Faculty of Engineering,

Research Unit Fiber Materials and Environmental Technologies (FibEnTech-UBI), Covilhã, Portugal

⁶⁾ University Nova of Lisbon, Faculty of Science and Technology, Center for Environmental and Sustainability Research (CENSE), Lisbon, Portugal

⁷⁾ Stalprodukt S.A., Bochnia, Poland

RECEIVED 10.05.2022

ACCEPTED 26.09.2022

AVAILABLE ONLINE 31.12.2022

Abstract: The aim of the study was to model the operation of a wastewater treatment plant using the Monte Carlo method and selected probability distributions of random variables. Pollutant indices in treated wastewater were analysed, such as: biological oxygen demand (BOD_5), chemical oxygen demand (COD_{Cr}), total suspended solids (TSS), total nitrogen (N_{tot}), total phosphorus (P_{tot}). The preliminary analysis of pollution indicators series included the: calculation of descriptive statistics and assessment of biological degradability of wastewater. The consistency of the theoretical distributions with the empirical ones was assessed using Anderson–Darling statistics. The best-fitting statistical distributions were selected using the percent bias criterion. Based on the calculations performed, it was found that the analysed indicators of pollution in treated wastewater were characterised by an average variability of composition for BOD_5 , COD_{Cr} and TSS, and a high variability of composition for N_{tot} and P_{tot} . The best fitted distribution was log-normal for BOD_5 , TSS, N_{tot} and P_{tot} and general extreme values for COD_{Cr} . The simulation carried out using the Monte-Carlo method confirmed that there may be problems associated with the reduction of nutrients (N_{tot} and P_{tot}) the analysed wastewater treatment plant. Results of values obtained of the risk values of negative control of wastewater treatment plant operation for biogenic compounds, different from 1, indicate that the number of exceedances at the outflow may be higher than the acceptable one.

Keywords: efficiency, mathematical simulation, Monte Carlo simulation, pollution indicators, probability distribution, sewage treatment plant



INTRODUCTION

In recent years, an increasing rate of industrialisation, urbanisation and population growth can be observed, which directly translates into increased environmental pollution. For this reason, sewage treatment has become one of the most important environmental issues [Angelakis, Gikas 2014; Flores-Alsina et al. 2010; Psarrou et al. 2018; Taheriyoun, Moradinejad 2015]. Due to the variety of sewage treatment processes, use of chemical reagents, production of sludge and emission of gases, it is very important to carefully analyse the reliability of pollutant removal. The evaluation of the performance of wastewater treatment plants includes many individual objectives, with the most important being the removal of pollutants that may cause changes in the aquatic environment [FLORES-ALSINA et al. 2008]. Therefore, the selected alternative method needs to comply with current regulations as well as to minimise the environmental impact on the receiving water body [Gernaey et al. 2004; Gizińska-Górna et al. 2017; MŁYŃSKI et al. 2018; TODESCHINI 2016]. Therefore, it is important to study the quality of wastewater after the treatment process, which allows verification of the operation of individual facilities of the process line and the final effect of treatment in relation to the applicable guidelines.

There are many studies concerning the disposal of municipal wastewater at selected treatment plants. They concern both the analysis of wastewater treatment efficiency [Bugajski et al. 2019; Hendren et al. 2013; Kurek et al. 2020; Młyński et al. 2016a, b; Taheriyoun, Moradinejad 2015] and the susceptibility of wastewater to biological decomposition processes [MŁYŃSKA et al. 2017; Nowobilska-Majewska, Bugajski 2019; Płuciennik-Korop-CZUK, JAKUBASZEK 2012]. The studies and results presented here provide important information for plant operators. The proper functioning of wastewater treatment plants is evidenced by their reliability, which means the plant's capacity to discharge of the expected amount of sewage, to the extent required by their receiver under certain operating conditions of the plant, within the assumed operating time and with random changes in the functional characteristics of the plant [DŁUGOSZ, GAWDZIK 2013]. One of the most commonly used methods of assessing the performance of wastewater treatment plants is to determine reliability indices of the facility's operation by comparing the values of pollutant indices in the effluent (arithmetic mean) to the permissible values set out in the relevant Regulation. The presented assumption is highly simplified, as it is based only on an empirical sample of analysed random variables [MŁYŃSKI et al. 2019]. According to Olyaei et al. [2018], the reliability of wastewater treatment plant operation is mainly considered as the percentage of time in which the expected concentration of pollutants in the effluent at the outfall meets the standards. Based on the variability of the pollutant indicators in the treated effluent, mathematical simulations of the operation of wastewater treatment plants can be carried out, which provides a more complete picture of the assessment of the plant's performance compared to a model based solely on short observation series for the given indicators. Simulations of wastewater treatment plant operation are often considered research problems, both at the stage of design, modernisation or operation of such facilities. However, the research so far with the use of mathematical models focuses on the simulation of pollutant values at the outflow with an assumed specific form of probability distribution, e.g. Weibull

[Bugajski et al. 2016; Marzec 2017]. It should be emphasised that this represents a large generalisation because pollution indicators have an underdetermined character. Hence, it can be used more than one probability distribution to described them. To do this, the best-fitting functions should be selected from the group of distributions [Młyński et al. 2019]. Monitoring the operation of wastewater treatment plants with the use of probability distributions allows to present the analysed phenomenon in a wider range, thanks to modelling complex dependencies and forecasting their values at a given time. The simulation values obtained can be helpful in assessing the risk of improper functioning of the treatment plant.

With this in mind, the paper presents a simulation of the operation of a wastewater treatment plant using selected statistical distributions, which has allowed the determination of the size of reliability indexes of the facility's operation based on the predicted values, reflecting the factors affecting the operation of the facility.

MATERIALS AND METHODS

STUDY OBJECT

Reliability analysis was carried out on the example of a sewage treatment plant located in the city of Tarnów in southern Poland. A mechanical-biological wastewater treatment plant is a facility that uses active sludge in the biological phase to remove pollutants from wastewater. The plant receives wastewater from individual users as well as industrial wastewater. Total population equivalent for wastewater treatment plant is 460 000. Design capacity of the treatment plant is 86 400 m³·d⁻¹. The process of mechanical wastewater treatment includes: screens, horizontal sand traps, radial primary clarifiers. The process of biological decomposition of pollutants takes place in biological reactors with active sludge, followed by the phase of clarification of treated sewage in secondary radial settling tanks. At the end of the process, the treated wastewater is discharged to the receiving body, i.e. Biała.

DATA USED

The analysis was made on the basis of the data covering the values of pollution indicators: BOD₅ (biochemical oxygen demand), COD_{Cr} (chemical oxygen demand), total suspended solids (TSS), total nitrogen (Ntot) and total phosphorus (Ptot) in the period from 2015 to 2020 for raw and treated sewage. The analysis was made according to the following stages: preliminary statistical analysis of the series of pollutant indicators, assessment of the susceptibility of wastewater to biological decomposition processes, selection of the theoretical function, best-fit empirical distribution, modelling of the values of pollutant indicators in raw and treated wastewater, determination of the values of reliability indicators of the operation of the treatment plant based on the results of the simulation carried out. According to the current Rozporządzenie [2019] for wastewater treatment plants serving more than 100,000 PE, the permissible values for: BOD_5 – 15 mg $O_2 \cdot dm^{-3}$, $COD_{Cr} - 125 \text{ mg } O_2 \cdot dm^{-3}$, $TSS - 35 \text{ mg } O_2 \cdot dm^{-3}$, N_{tot} - 10 mg·dm⁻³, P_{tot} - 1 mg·dm⁻³. The required number of control samples is 24 samples per year, while the number of noncompliant samples is 3. Samples of raw and treated sewage for physicochemical tests were collected twice a month, in the period from 2015 to 2020, which gives 159 samples of raw and 159 samples of treated sewage that were analysed in the period from 2015 to 2020.

PRELIMINARY ANALYSIS OF POLLUTION INDICATORS

The preliminary analysis of pollution indicators series included the calculation of descriptive statistics. The following descriptive statistics were determined: mean (mean), minimum (min) and maximum (max) values; measures of dispersion – standard deviation (SD) and coefficient of variation (C_v).

ASSESSMENT OF BIOLOGICAL DEGRADABILITY OF WASTEWATER

The assessment of the susceptibility of wastewater inflow to the treatment plant in Tarnów and after the biological treatment process was made on the basis of the ratio of COD_{Cr} to BOD_5 . For this purpose, the mean annual values of both indicators of the content of organic compounds were compared. This assessment was based on the relationships available in the literature describing the measures of biodegradability of organic pollutants present in wastewater according to the following relationships [KLIMIUK, ŁEBKOWSKA 2003; MIKSCH, SIKORA 2010]:

$$COD_{Cr}/BOD_5 < 2.0$$
 easy biodegradability (1)

$$COD_{Cr}/BOD_5 = 2.0-2.5$$
 mean bio-degradability (2)

$$COD_{Cr}/BOD_5 = 2.5-5.0$$
 low biodegradability (3)

$$COD_{Cr}/BOD_5 > 5.0$$
 non-degradable matter (4)

Additionally, based on the mean annual concentrations of $N_{\rm tot}$ and $P_{\rm tot}$ both in the sewage before entering the technological system and after treatment, the susceptibility of the wastewater to the processes of biological removal of biogenic compounds was determined. For this purpose, quotients of the values of $N_{\rm tot}/BOD_5$ and $P_{\rm tot}/BOD_5$ were determined and then compared to the values reported in the literature, which indicate that denitrification (5) and defosphatation (6) processes occur most effectively when [Heidrich, Witkowski 2015; Janosz-Rajczyk 2008; Łomotowski, Szpindor 1999]:

$$N_{\text{tot}}/BOD_5 < 0.25 \tag{5}$$

$$P_{\text{tot}}/BOD_5 < 0.04 \tag{6}$$

THEORETICAL DISTRIBUTION FITTING

Before the Monte Carlo simulation, a three-stage intermediate analysis was performed in the analysis: adjusting the theoretical distribution to empirical distributions, assessing the consistency of the theoretical distributions with the empirical distributions of the analysed pollution indicators and selecting the best-fitted theoretical distributions.

On the basis of observation series of the analysed indicators of pollution in raw and treated wastewater, an attempt was made to determine the best-fitting theoretical distributions to the empirical distributions of the random variables under study. The following theoretical distributions were analysed: Johnson SU,

Weibull, Gaussian mixture model (GMM), general extreme values (GEV), half-normal distribution, log-normal, Pareto, normal distribution, triangular, Rayleigh. The distributions of each distribution f(x) are described by the following functions [Alam et al. 2018; GLICKMAN, XU 2008; JAGIEŁŁO et al. 2016; WAŁĘGA et al. 2014; WANG (ed.) 2010; Yu et al. 2012; ZORAN, WEISS 2011]:

• Johnson SU distribution:

$$f(x;\xi;\lambda;\gamma;\delta) = \frac{\delta}{\sqrt{2\pi}} \frac{\lambda}{(\xi+\lambda-x)(x-\xi)}$$

$$\exp\left(-\frac{1}{2}\left[y+\delta\ln\left(\frac{x-\xi}{\xi+\lambda-x}\right)\right]^2$$
(7)

where: x = variable, $\lambda = \text{scale parameter}$, y, $\delta = \text{shape parameter}$, $\xi = \text{location parameter}$.

• Weibull distribution:

$$f(x) = \left(\frac{\kappa}{\alpha}\right) \left(\frac{x}{\alpha}\right)^{\kappa - 1} e^{-\left(\frac{x}{\alpha}\right)^{\kappa}} \tag{8}$$

where: α = scale parameter, κ = shape parameter.

• GMM distribution:

$$f(x) = \sum_{k=1}^{K} \omega_k N_P(x, 0_P, \Sigma_k)$$
 (9)

where: K = number of components, $N_p(0_p, \Sigma_k) =$ multi-variate Gaussian distribution, $\omega_k =$ weights such that $\Sigma_k \omega_k = 1$.

• GEV distribution:

$$f(x) = \alpha^{-1} e^{[-(1-\kappa)y - e^{-y}]}$$
 (10)

$$y = -\kappa^{-1} \log \left(1 - \frac{\kappa(x - \xi)}{\alpha} \right), \ \kappa \neq 0$$
 (11)

$$y = \frac{\mathbf{x} - \xi}{\alpha}, \ \kappa = 0 \tag{12}$$

where: κ = shape parameter, α = scale parameter, ξ = location parameter.

• Half-normal distribution

$$f(x) = \frac{2}{\sqrt{2\pi\sigma}} \exp\left(-\frac{x^2}{2\sigma^2}\right) \text{ if } x \ge 0$$
 (13)

where: σ = variance value.

· Log-normal distribution

$$f(x) = \frac{1}{x\sigma_Y \sqrt{2\pi}} e^{\left[-\frac{1}{2\sigma_Y^2} (\ln(x) - \mu_Y)^2 \right]}$$
(14)

where: μ = mean value, σ^2 = variance value.

• Pareto distribution

$$f(x) = \frac{c}{x^{c+1}} \tag{15}$$

where: c = shape parameter.

• Normal distribution

$$f(x) = \frac{1}{\sigma\sqrt{2\pi}} e^{\left[-\frac{1}{2\sigma^2}(x-\mu)^2\right]}$$
 (16)

• Triangular distribution

$$f(x) = \begin{cases} \frac{2(x-a)}{(b-a)(m-a)} & \text{for } a \le x \le m\\ \frac{2(b-x)}{(b-a)(m-a)} & \text{for } m \le x \le b \end{cases}$$
 (17)

where: m = mode, a = lower limit, b = upper limit.

· Rayleigh distribution

$$F(x;\lambda) = 1 - e^{-(\lambda x)^2}, \ x \ge 0, \lambda > 0$$
 (18)

$$f(x;\lambda) = 2\lambda^2 x e^{-(\lambda x)^2}, x \ge 0, \lambda > 0$$
(19)

where: λ = scale parameter.

The Anderson–Darling (A–D) test was used to assess the compliance of the theoretical distributions with the empirical distributions of the analysed pollution indicators. The A–D statistic is more sensitive over the entire range of distributions and is more likely to detect differences between distributions, thats it gives a better estimate of compliance than the other tests. The hypotheses for the A–D test are as follows: H₀ data has a specific distribution, H₁ data does not have a specific distribution. The Anderson–Darling statistics is described by the correlations [Evans *et al.* 2017; Jäntschi, Bolboacă 2018; Zeng *et al.* 2015]:

$$A-D=-n-\sum_{i=1}^{n}\frac{(2i\ -\ 1)\ln[F(X_{i})]\ +\ \ln[1\ -\ F(X_{n+1-i})]}{n} \eqno(20)$$

where: n = observations number, F = theoretical cumulative distribution, $F_n =$ empirical cumulative distribution.

Since the critical values for the A–D statistic depend on the type of probability distribution being tested, hence the verification of the hypotheses was based on the p-value of the test. If the p-value was less than $\alpha=0.05$, then rejected the null hypothesis that the data come from that distribution.

SELECTION OF BEST-FIT THEORETICAL DISTRIBUTIONS

If random variables could be described by more than one theoretical distribution, an identification of the best-fitting theoretical distributions to the empirical distributions was made using the percent bias criterion (*PBIAS*), which describes the Equation (21) [AJMAL *et al.* 2020]:

$$PBIAS = \frac{\sum_{i=1}^{n} (Q_{oi} - Q_{ei})}{\sum_{i=1}^{n} Q_{oi}} 100$$
 (21)

where: Q_{oi} = observed values, Q_{ei} = estimated values.

The *PBIAS* describes the average tendency of estimated values to be greater or less than their observed values. The closer the *PBIAS* value is to 0% the greater the agreement of the compared quantities, positive values indicate underestimation of the model and negative values indicate overestimation [Gupta et al. 1999; Moriasi et al. 2007; Yuan et al. 2014]. According to

guidelines, *PBIAS* value $< \pm 10\%$ indicates a very good fit; $\pm 10\%$ $\le PBIAS < \pm 15\%$ – good; $\pm 15\% \le PBIAS < \pm 25\%$ – satisfactory; and *PBIAS* $\ge \pm 25\%$ – unsatisfactory fit [Archibald *et al.* 2014; Donigian *et al.* 1983; Moriasi *et al.* 2007; Singh *et al.* 2004; Van Liew *et al.* 2003].

MODELLING OF POLLUTANT INDICATOR VALUES

Modelling of the values of pollutant indices in treated wastewater was carried out using the Monte-Carlo method. It is defined as the use of a sequence of random numbers to build a sample from a hypothetical population, from which it is possible to determine statistical estimators of the parameters of the problem sought to be solved [Halton 1970]. In practice, it consists in the formulation of a stochastic model describing a real phenomenon, and then repeated implementation of this model using randomly generated variables (according to the assumed probability distribution) and statistical analysis of the obtained results. The Monte-Carlo method is widely used in analyses related to the operation of water and wastewater management facilities, as indicated by the works of Hendren et al. [2013], Schauberger et al. [2013], Barton et al. [2015]. In this study, simulations were performed for best-fit statistical distributions describing the empirical distributions of the analysed pollution indicators. One simulation cycle consisted of the generation of 365 random values of pollution indicators. For each generated set, a 24-element sample was determined at random. In each cycle, the number of observations from the simulation which did not meet the criterion of technological effectiveness (i.e. where the value from the simulation was greater than the permissible value) was determined and the result of the control of operation of the treatment plant was determined (control indicator (CF) = 1 when the treatment plant was assessed negatively, i.e. when the number of samples not meeting the requirements exceeded the permissible value of the number of samples which may not meet such requirements; CF = 0 otherwise). Each of the simulation cycle was then repeated 100 times.

DETERMINATION OF RELIABILITY COEFFICIENTS FOR THE OPERATION OF TREATMENT PLANTS

On the basis of the simulation carried out, the reliability estimators of the operation of the wastewater treatment plant were determined: the technological efficiency index R, the reliability coefficient CR and the risk of negative control of the operation of the wastewater treatment plant R_e . Calculations were carried out according to the formulas [Andraka, Dzienis 2013]:

$$R = \frac{N_{\text{sym}} - N_{X\text{perm}}}{N_{\text{sym}}} \tag{22}$$

where: N_{sym} = number of simulations carried out, $N_{X\text{perm}}$ = number of samples from simulations not meeting the limit values for the pollutant indicator analysed:

$$CR = \frac{m_x}{N_{\text{perm}}} \tag{23}$$

where: m_x = the average value of a given indicator for the simulations carried out, N_{perm} = the limit value of the indicator concerned.

$$R_e = \frac{\sum_{i=1}^{N_{\text{sym}}} CF_i}{N_{\text{sym}}} \tag{24}$$

where: $\sum_{i=1}^{N_{\mathrm{sym}}} CF_i$ = the sum of the results of the plant operation checks for the simulations carried out, N_{sym} = number of simulations carried out.

RESULTS AND DISCUSSION

GENERAL INFORMATION

The analysis of the results carried out in this study includes preliminary analysis of the data, assessment of the susceptibility of wastewater to biological decomposition processes, selection of the best-fitting theoretical to empirical distribution, modelling of the values of pollutant indices in raw and treated wastewater, together with an analysis of the correctness of the two-stage treatment processes.

PRELIMINARY DATA ANALYSIS

In order to characterise the dynamics of changes in the quality of raw and treated sewage, descriptive statistics were determined: location and dispersion measures for the analysed values of pollutant indexes in sewage. The results of the analyses are presented in Table 1.

The results of the preliminary analysis are presented in Table 1. Based on it, it was found that in the raw sewage the difference between the extreme values (min. and max.) was respectively: for BOD_5 – 85%, COD – 85%, TSS – 88%, N_{tot} – 76%, P_{tot} – 88%. Additionally, it can be concluded that the composition

Table 1. Preliminary data analysis for observation series of pollutant indicators in raw and treated wastewater

| D.H. d. P. d. | Min. | Mean | Max. | SD | <i>a</i> () | | | |
|---------------------|--------|-------------|---------|--------|-------------|--|--|--|
| Pollutant indicator | | C_{v} (-) | | | | | | |
| Raw sewage | | | | | | | | |
| BOD_5 | 140.00 | 518.93 | 940.00 | 182.55 | 0.35 | | | |
| COD_{Cr} | 266.00 | 970.67 | 146.00 | 294.37 | 0.30 | | | |
| TSS | 140.00 | 476.43 | 1200.00 | 165.62 | 0.35 | | | |
| N _{tot} | 36.40 | 80.09 | 149.00 | 18.23 | 0.23 | | | |
| P _{tot} | 2.93 | 7.56 | 24.50 | 2.64 | 0.35 | | | |
| Treated sewage | | | | | | | | |
| BOD_5 | 3.50 | 7.90 | 23.00 | 3.27 | 0.41 | | | |
| COD_{Cr} | 27.50 | 69.41 | 146.00 | 20.26 | 0.29 | | | |
| TSS | 6.80 | 15.07 | 35.00 | 5.81 | 0.39 | | | |
| N _{tot} | 3.30 | 10.57 | 50.60 | 8.06 | 0.76 | | | |
| P _{tot} | 0.21 | 0.59 | 2.00 | 0.31 | 0.52 | | | |

Explanations: SD = standard deviation, $C_{\nu} = \text{coefficient of variation}$, $BOD_5 = \text{oxygen demand}$, $COD_{Cr} = \text{chemical oxygen demand}$, TSS = total suspended solids.

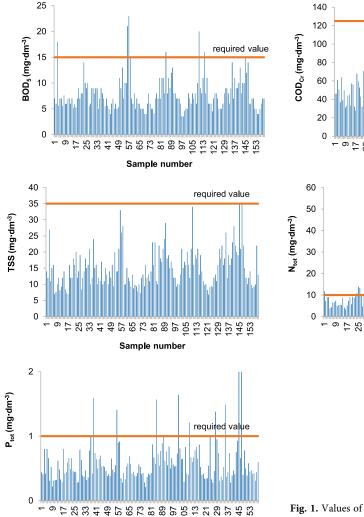
Source: own study.

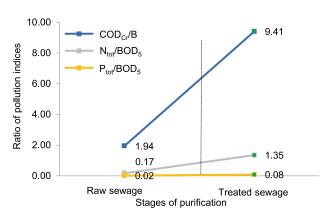
of wastewater flowing into the Tarnów WWTP did not differ significantly from the typical composition for domestic wastewater from southern Poland, as indicated by the results of studies conducted by Kaczor [2009] and Młyński et al. [2020]. The values of coefficient of variation (C_v) for individual indicators of pollution indicate an average variability of the composition of wastewater flowing into the treatment plant. In the case of treated wastewater, the characteristic values of pollutant indicators are presented in Table 1 and Figure 1. The disproportion between the values of pollutants from the organic group was respectively for BOD_5 – 84% and COD – 81%, and for TSS was at the level of 81%. For biogenic compounds, the differences were at the level of 93% for Ntot and 90% for Ptot In the case of organic pollutants in the period of analysis (2015-2020) exceedances of limit values can be observed. For BOD5 there were 8 exceedances of the limit value (15 mg O_2 ·dm⁻³) and for *COD* there were two observations above 125 mg O₂·dm⁻³. For TSS, two samples were recorded throughout the observation period that did not meet the limit value of 35 mg·dm⁻³. In the case of biogenic compounds as many as 31 exceedances of the standard values (10 mg·dm⁻³) were recorded for total nitrogen in treated wastewater and 10 samples which exceeded the permissible value of total phosphorus concentration in the outflow (1 mg·dm⁻³).

The recorded exceedances of values of pollutant indices could have been caused by a varied course of factors which significantly shape the effectiveness of treatment processes, i.e. the volume of wastewater inflow or the temperature of wastewater. Another factor may be the size of the pollutant load in the effluent entering the plant. The quality and quantity of wastewater is strongly influenced by the nature of urban households. People, who did not have access to the sewage treatment are accustomed to saving water. The result is the concentration of higher amount of pollutants in less water used. In addition, trends in household water consumption result in high heterogeneity in wastewater discharge, which exposes treatment systems to unstable operation [WAŁĘGA et al. 2018]. Analysing the value of the C_{ν} values, we can observe an average variability in the composition of organic pollutants and total suspended solids, and a high variability in the composition for total nitrogen and total phosphorus at the outflow in wastewater after biological treatment processes. As shown by the results of studies conducted by other authors, this is a characteristic feature of treated wastewater [Bugaiski et al. 2016; Chmielowski et al. 2017; Kaczor et al. 2015; Młyński et al. 2020; Wasik, Chmielowski 2013].

ASSESSMENT OF BIOLOGICAL DEGRADABILITY OF WASTE WATER

Based on the presented relationships (Eqs. (1)–(4)), it can be concluded that in the period from 2015 to 2020, wastewater flowing to the treatment plant in Tarnów was characterised by high susceptibility to decomposition of organic compounds, as evidenced by the value of the $COD_{\rm Cr}/BOD_5$ quotient (Fig. 2). The value of the quotient $COD_{\rm Cr}/BOD_5$ after the process of biological treatment at the level of 9.41 indicates a high degree of decomposition of organic compounds and organic substances difficult to decompose remaining in the sewage. The quotient values between $COD_{\rm Cr}$ and BOD_5 indicate that the processes of organic pollutants removal in the analysed two-stage system are correct. Analysing the susceptibility of raw sewage to the

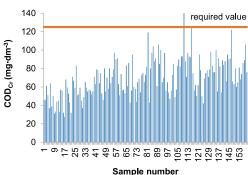




Sample number

Fig. 2. A susceptibility of the raw sewage and sewage after biological process on biodegradability based on $COD_{\rm Cr}/BOD_5$, $N_{\rm tot}/BOD_5$ and $P_{\rm tot}/BOD_5$ ratios; BOD_5 , $COD_{\rm Cr}$ as in Tab. 1; source: own study

decomposition of biogenic compounds, it can be observed that the determined relations between $N_{\rm tot}$ and BOD_5 and between $P_{\rm tot}$ and BOD_5 provided appropriate conditions for the processes of biological decomposition of nitrogen compounds by denitrification, and the removal of phosphorus compounds by defosfatation. After the process of biological treatment of wastewater, an increase in the values of the determined ratios can be observed,



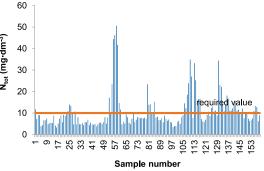


Fig. 1. Values of analysis indicators of pollution in treated sewage against permissible values; BOD_5 , COD_{Cr} , TSS as in Tab. 1; source: own study

which indicates a significant decrease in the amount of biologically degradable substances and in the effectiveness of the processes of removing biogenic compounds (Fig. 2). Similar results with observations were also noted in the work by Maynska et al. [2017], where the authors obtained very similar values of quotients of analysed indicators of pollution. The obtained quotient values indicate the proper functioning of the mechanical-biological treatment plant in Tarnów.

VERIFICATION OF THEORETICAL PROBABILITY DISTRIBUTIONS

The fit of the theoretical distributions to the empirical distributions of the observational series of the pollution indicators analysis in raw and treated sewage was determined by the following functions: Johnson SM, Weibull, GMM, GEV, halfnormal, log-normal, normal, Pareto, triangular, Rayleigh. The fit of the distributions was assessed using the Anderson–Darling test for a fixed significance level of $\alpha=0.05$. When the obtained p-value was below the assumed significance level, it was found that the theoretical functions did not match the empirical functions for the analysed indicator measurement series. The obtained results of the analysis are presented in Table 2.



Table 2. The value of fitting the analysed theoretical distributions to the empirical distributions of random variables of pollution indicators in treated sewage

| Distribution | BOD_5 | | COD_{Cr} | | TSS | | N _{tot} | | P _{tot} | |
|--------------|---------|-------|---------------------|-------|--------|-------|------------------|-------|------------------|-------|
| | A-D | p | A-D | p | A-D | p | A-D | p | A-D | p |
| GMM | 1.352 | 0.215 | 0.297 | 0.939 | 0.829 | 0.459 | 1.535 | 0.168 | 1.877 | 0.107 |
| GEV | 1.078 | 0.318 | 0.357 | 0.889 | 0.708 | 0.551 | 0.181 | 0.994 | 0.323 | 0.920 |
| Johnson SU | - | ı | 0.370 | 0.877 | 0.461 | 0.785 | _ | İ | - | ı |
| Log-normal | 1.849 | 0.111 | 0.636 | 0.614 | 0.817 | 0.468 | 2.060 | 0.085 | 1.278 | 0.239 |
| Normal | 6.985 | 0.000 | 0.646 | 0.605 | 3.533 | 0.014 | 14.286 | 0.000 | 8.315 | 0.000 |
| Weibull | 6.359 | 0.001 | 0.855 | 0.442 | 2.995 | 0.027 | 7.639 | 0.000 | 6.022 | 0.001 |
| Triangular | 24.397 | 0.000 | 6.131 | 0.000 | 7.188 | 0.000 | 63.176 | 0.000 | 33.275 | 0.000 |
| Rayleigh | 9.490 | 0.000 | 14.474 | 0.000 | 7.438 | 0.000 | 16.041 | 0.000 | 6.097 | 0.001 |
| Pareto | 23.211 | 0.000 | 24.841 | 0.000 | 18.186 | 0.000 | 14.468 | 0.000 | 18.766 | 0.000 |
| Semi-normal | 26.244 | 0.000 | 34.700 | 0.000 | 25.137 | 0.000 | 11.102 | 0.000 | 18.129 | 0.000 |

Explanations: BOD_5 , COD_{Cr} , TSS as in Tab. 1, A-D = Anderson-Darling statistics, p-value is above the adopted significance level $\alpha = 0.05$ (theoretical distribution can be applied), GMM = Gaussian mixture model, GEV = general extreme values. Source: own study.

Based on the results presented in Table 2, it was found that the observation series for the BOD5 indicator can be described by the three probability distributions analysed (GMM, GEV, log-normal). This is indicated by the *p*-value values for the A–D test statistics taking values above the assumed significance level $\alpha = 0.05$. For the second indicator from the organic group (COD_{Cr}) , the A-D test showed that a p-value above 5% occurs for the distributions: GMM, GEV, Johnson SU, log-normal, Normal, Pareto, Weibull, which means that the hypothesis H₀, stating that the theoretical distribution analysed is consistent with the empirical distribution, should be accepted. With regard to total suspended solids, a p-value above 0.05 indicates the possibility of using the distributions: GMM, GEV, Johnson SU. On the other hand, in the case of indexes from the biogenic group (Ntot and Ptot), statistical analysis showed the conformity of the theoretical distribution with the empirical distribution of these indices for the functions: GMM, GEV, log-normal.

SELECTION OF THE BEST-FITTING STATISTICAL DISTRIBUTION

Based on the analysis of the matching of the theoretical and empirical distributions, it was found that the observation series of pollution indicators in treated wastewater can be described by more than one theoretical distribution. Calculations were performed only for those functions for which the A-D criterion confirmed their conformity with the empirical distribution of random variables. The selection of the best-fitting distribution was made on the basis of the PB criterion and the quantilequantile fit plot. The results of the analyses are summarised in Table 3, and Figure 3 shows the best-fit statistical distributions.

Based on the results presented in Table 3, it was found that for BOD5, the best-fitting theoretical distribution is the lognormal distribution, for which the PBIAS value is equal to -0.225%. For COD_{Cr} , the best-fitting theoretical distribution was found to be the GEV distribution, for which the PBIAS value is -0.094%. In the case of total suspended solids and biogenic

Table 3. Analysis of percent bias (PBIAS) for the pollutants under

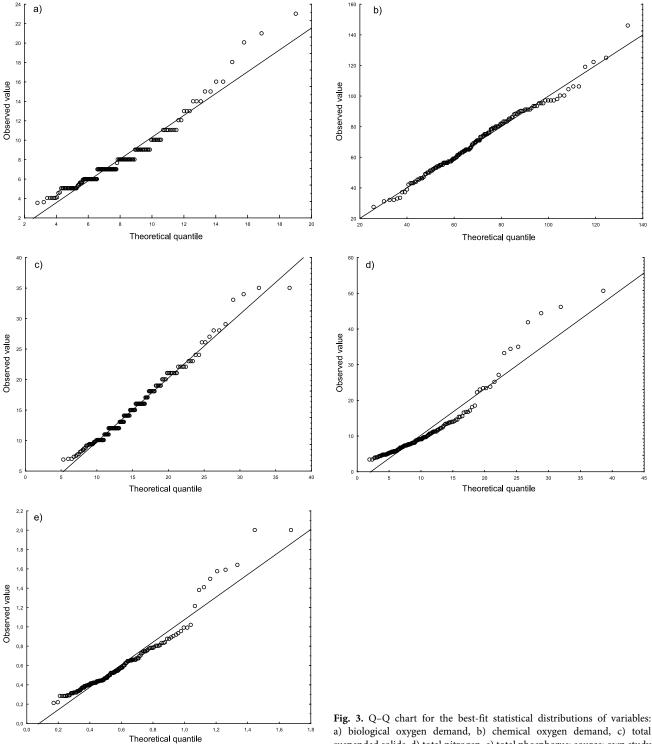
| Distribution | PBIAS (%) | | | | | | |
|--------------|-----------|------------|--------|------------------|------------------|--|--|
| Distribution | BOD_5 | COD_{Cr} | TSS | N _{tot} | P _{tot} | | |
| GMM | 0.171 | -0.024 | 0.233 | 1.409 | 0.481 | | |
| GEV | 0.005 | -0.094 | 0.094 | 0.015 | 0.050 | | |
| Johnson SU | - | -0.036 | 0.008 | - | - | | |
| Log-normal | -0.225 | -0.067 | -0.097 | -0.772 | -0.295 | | |
| Normal | - | 0.306 | - | - | - | | |
| Weibull | - | 1.025 | - | - | - | | |

Explanations: BOD5, CODCr, TSS as in Tab. 1, GMM, GEV as in Tab. 2, bold - best-fit statistical distributions.

Source: own study.

indicators (Ntot and Ptot), the best-fitting theoretical function is the log-normal distribution, which is confirmed by the PBIAS values. The analysis carried out allowed to explicitly indicate that for the indicator from the organic group (BOD₅), biogenic indicators and TSS the best-fitting theoretical distribution is lognormal. In the case of COD_{Cr} such a distribution turned out to be GEV, however the comparable value of the PBIAS criterion for the log-normal distribution indicates its applicability in the description of this indicator. Figure 3 presents the best-fitting statistical distributions describing the variables. The possibility of using the log-normal distribution to monitor the performance of wastewater treatment plants has been described in the works of numerous authors [Andraka, Dzienis 2013; Oliveira, von Sperling 2007; 2008], which indicate that it is the log-normal distribution that best captures the statistical variability of quality parameters in treated wastewater.

Karolina Migdał, Agnieszka Operacz, Iryna Vaskina, Paulina Śliz, Jorge Tavares, Adelaide Almeida, Michał Migdał



DETERMINATION OF RELIABILITY COEFFICIENTS FOR THE OPERATION OF THE TARNÓW WASTEWATER TREATMENT PLANT

Based on the results for the best-fitting statistical distributions, values for the pollution indicators were determined using the Monte-Carlo method. The modelling was based on the log-normal distribution for the indicators BOD5, TSS and biogenic group (Ntot and Ptot). For CODCr the simulation was based on the GEV distribution. On the basis of the simulated values, the indices of

a) biological oxygen demand, b) chemical oxygen demand, c) total suspended solids, d) total nitrogen, e) total phosphorus; source: own study

technological efficiency (R), reliability coefficient (CR) and risk of negative control of wastewater treatment plant operation (R_e) were calculated. The results are summarised in Table 4.

Based on the results of the simulation carried out, it can be concluded that the wastewater treatment plant in Tarnów is functioning properly in terms of the reduction of organic pollutants and total suspended solids. The values of technological efficiency index R for BOD5 and CODCr, as well as for TSS are very close to each other and equal to unity, which indicates that the number of samples that do not meet the requirements for the

Table 4. Efficiency indicators of wastewater treatment plant operation determined on the basis of Monte-Carlo simulation

| Reliability | Pollutant indicator | | | | | | | |
|-------------|---------------------|------------|-------|------------------|------------------|--|--|--|
| coefficient | BOD_5 | COD_{Cr} | TSS | N _{tot} | P _{tot} | | | |
| R | 0.967 | 0.967 | 0.998 | 0.674 | 0.929 | | | |
| CR | 0.520 | 0.553 | 0.429 | 0.999 | 0.575 | | | |
| R_e | 0.050 | 0.090 | 0.050 | 0.980 | 0.270 | | | |

Explanations: BOD_5 , COD_{Cr} , TSS as in Tab. 1, R = technological efficiency, CR = reliability coefficient, R_e = risk of negative control. Source: own study.

quality of treated wastewater may only occasionally occur during the operation of this wastewater treatment plant. This is confirmed by the values obtained for the reliability index of the operation of the CR treatment plant. The values of the risk indicator of negative control of the treatment plant R_e operation for all three indicators (BOD₅, COD_{Cr} and TSS) indicate that during the plant operation there may occur exceedances of the permissible values at the outlet, however, the number of such exceedances should not be higher than the one admissible for the treatment plant with PE (population equivalent) exceeding 100,000. The recorded exceedances will therefore be incidental phenomenon, without negative impact on the plant operation and environment. In case of biogenic pollutants, the simulation carried out showed that the WWTP may not meet the requirements related to reduction of total nitrogen and total phosphorus in relation to the normative levels. Moreover, the risk values of negative control of wastewater treatment plant operation for biogenic compounds, different from 1, indicate that the number of exceedances at the outflow may be higher than the acceptable one. The value of R_e for total nitrogen, which is equal to 1, is particularly unfavourable. It means that during the operation period the treatment plant will not function properly in terms of reduction of total nitrogen from the wastewater. The recorded excess of total nitrogen in the outflow may be caused by running the nitrification process at the temperature of wastewater below 12°C. As reported by BUGAJSKI and Kaczor [2008a], Bugajski and Mielenz [2008b] and Bugajski et al. [2015], the temperature of the wastewater has a significant impact on the processes of nitrogen compounds removal in those treatment plants whose technology is based on activated sludge. The cited authors also indicate a temperature of 12°C as the limiting temperature below which the nitrification process is significantly inhibited. Therefore, this explains the less favourable values of calculated reliability indices for the operation of the analysed wastewater treatment plant for total nitrogen, in comparison with other pollution indices. One of the suggested solutions for wastewater treatment plant operators, in order to increase the reduction of total nitrogen and total phosphorus from wastewater, could be the incorporation of biological beds into the existing technology. The process of nitrification will be carried out on the beds, while in the anaerobic and hypoxic chambers, operating in the activated sludge technology, the process of nitrogen and phosphorus removal will be carried out. The separation of nitrifying microorganisms from heterotrophic ones will eliminate the phenomenon of competition between these organisms and, as a result, increase the stability of the population of nitrifying bacteria [MŁYŃSKI et al. 2019; WAŁĘGA et al. 2019].

CONCLUSIONS

The aim of this study was to model the operation of a wastewater treatment plant, using the Monte-Carlo method. The pollutant indexes in the treated wastewater were analysed: BOD₅, COD_{Cr}, TSS, Ntot and Ptot. Simulations were performed using the following probability distributions: Johnson SU, Weibull, gaussian mixture model (GMM), general extreme values (GEV), halfnormal distribution, log-normal, Pareto, normal, triangular, Rayleigh. The fit of the theoretical distributions together with the empirical distributions was assessed using Anderson-Darling (A-D) statistics. The best-fitting statistical distributions were selected using the percent bias criterion (PBIAS) criterion. Based on the calculations performed, it was found that the analysed indicators of pollution in treated wastewater were characterised by an average variability of composition for organic pollutants and total suspended solids, and a high variability of composition for total nitrogen and total phosphorus. The best fitted distribution was log-normal for BOD5, TSS, Ntot and Ptot and GEV for COD_{Cr}. The simulation carried out using the Monte-Carlo method confirmed that there may be problems associated with the reduction of nutrients at the analysed wastewater treatment plant. The Monte Carlo simulation method is a useful tool for modelling the technological reliability of a wastewater treatment plant, provided that an appropriate theoretical distribution for the random variables is indicated. This makes it possible to generate new data while retaining existing correlation structures between variables. The values generated in this way can be useful in assessing the performance of the treatment plant and in preparing various possible scenarios of its operation.

REFERENCES

AJMAL M., WASEEM M., KIM D., KIM T.W. 2020. A pragmatic slope-adjusted curve number model to reduce uncertainty in predicting flood runoff from steep watersheds. Water. Vol. 12(5) p. 1469–1484. DOI 10.3390/w12051469.

ALAM M.A., EMURO K., FARNHAM C., YUAN J. 2018. Best-fit probability distributions and return periods for maximum monthly rainfall in Bangladesh. Climate. Vol. 6 p. 9–25. DOI 10.3390/cli6010009.

Angelakis A.N., Gikas P. 2014. Water reuse: Overview of current practices and trends in the world with emphasis on EU states. Water Utilization Journal. Vol. 8 p. 67–78.

Andraka D., Dzienis L. 2013. Modelowanie ryzyka w eksploatacji oczyszczalni ścieków [Modeling of risk in the operation of wastewater treatment plants]. Roczniki Ochrony Środowiska. Vol. 15 p. 1111–1125.

ARCHIBALD J.A., BUCHANAN B., FUKA D.R., GEORGAKAKOSC B., LYON S.W., WALTER M.T. 2014. A simple, regionally parameterized model for predicting nonpoint source areas in the northeastern US. Journal of Hydrology: Regional Studies. Vol. 1 p. 74–91. DOI 10.1016/j. ejrh.2014.06.003.

Barton L.E., Auffan M., Durenkampf M., McGrath S., Bottero J., Wiesner M.R. 2015. Monte Carlo simulations of the transformation and removal of Ag, TiO₂, and ZnO nanoparticles in wastewater treatment and land application of biosolids. Science Total Environment. Vol. 511 p. 535–543. DOI 10.1016/j. scitotenv.2014.12.056.

BUGAJSKI P., ALMEIDA M.A.A., KUREK K. 2016. Reliability of sewage treatment plants processing sewage from school buildings located

Karolina Migdał, Agnieszka Operacz, Iryna Vaskina, Paulina Śliz, Jorge Tavares, Adelaide Almeida, Michał Migdał

- in non-urban areas. Infrastructure and Ecology of Rural Areas. Vol. 4 p. 1547–1557. DOI 10.14597/infraeco.2016.4.3.115.
- BUGAJSKI P., CHMIELOWSKI K., WĄSIK E. 2015. Wpływ opadów atmosferycznych na temperaturę oraz objętość ścieków w małym systemie kanalizacyjnym [Influence of atmospheric precipitations on temperature and volume of wastewater in small sewage system]. Infrastruktura i Ekologia Terenów Wiejskich. Vol. 4 p. 1057–1066. DOI 10.14597/infraeco.2015.4.1.084.
- BUGAJSKI P., KACZOR G. 2008a. Ocena działania wybranych przydomowych oczyszczalni w warunkach zimowych i letnich [Evaluation of operation of some domestic sewage treatment plants under winter and summer conditions]. Przemysł Chemiczny. Vol. 87(5) p. 424–426.
- BUGAJSKI P., KUREK K., MŁYŃSKI D., OPERACZ A. 2019. Designed and real hydraulic load of household wastewater treatment plants. Journal of Water and Land Development. Vol. 40 p. 155–160. DOI 10.2478/jwld-2019-0017.
- BUGAJSKI P., MIELENZ B. 2008b. Ocena pracy oczyszczalni ścieków w Wadowicach przed modernizacją [The assessment of working sewage treatment plant at Wadowice before moddernization]. Infrastruktura i Ekologia Terenów Wiejskich. Vol. 2 p. 129–138.
- Chmielowski K., Wasik E., Operacz A., Bugajski P., Kaczor G., Jurík L. 2017. Analysis of sewage susceptibility to biodegradation on an example of sewage treatment plant in Wodzisław Śląski. Infrastructure and Ecology of Rural Areas. Vol. 4(1) p. 1427–1443. DOI 10.14597/infraeco.2017.4.1.109.
- Długosz J., Gawdzik J. 2013. Ocena skuteczności funkcjonowania oczyszczalni ścieków w Barczy (woj. świętokrzyskie) [Evaluation of functioning of sewage treatment plant near Barcza (Świętokrzyskie Province)]. Proceedings of ECOpole. Vol. 7(1) p. 311–317. DOI 10.2429/proc.2013.7(1)042.
- Donigian A.S., Imhoff J.C., Bicknell B.R. 1983. Predicting water quality resulting from agricultural nonpoint-source pollution via simulation: HSPF. In: Agricultural management and water quality. Ames. Iowa State University Press p. 200–249.
- EVANS D.L., DREW J.H., LEEMIS L.M. 2017. The distribution of the Kolmogorov–Smirnov, Cramer–von Mises, and Anderson–Darling test statistics for exponential populations with estimated parameters. Computational Probability Applications. Vol. 247 p. 165–190. DOI 10.1080/03610910801983160.
- Flores-Alsina X., Gallego A., Feijoo G., Rodriguez-Roda R. 2010. Multiple-objective evaluation of wastewater treatment plant control alternatives. Journal of Environmental Management. Vol. 91(5) p. 1193–1201. DOI 10.1016/j.jenvman.2010.01.009.
- FLORES-ALSINA X., RODRIGUEZ-RODA I., SIN G., GERNAEY K.V. 2008. Multicriteria analysis of wastewater treatment plant control strategies under uncertainty. Water Resources. Vol. 42(17) p. 4485–4497. DOI 10.1016/j.watres.2008.05.029.
- Gernaey K.V., Van Loosdrecht M.C.M., Henze M., Lind M., Jørgensen S.B. 2004. Activated sludge wastewater treatment plant modelling and simulation: state of the art. Environmental Modelling & Software. Vol. 19 p. 763–783. DOI 10.1016/j.envsoft.2003.03.005.
- GIZIŃSKA-GÓRNA M., JÓŻWIAKOWSKI K., MARZEC M., PYTKA A., SOSNOWSKA B., RÓŻAŃSKA-BOCZULA M., LISTOSZ A. 2017. Analysis of the influence of a hybrid constructed wetland wastewater treatment plant on the water quality of the receiver. Annual Set The Environment Protection. Vol. 19 p. 370–393.
- GLICKMAN T.S., Xu F. 2008. The distribution of the product of two random variables. Statistics and Probability Letters. Vol. 78 p. 2821–2826. DOI 10.1016/j.spl.2008.03.031.
- GUPTA H.V., SOROOSHIAN S., YAPO P.O. 1999. Status of automatic calibration for hydrologic models: Comparison with multilevel

- expert calibration. Journal of Hydrology Engineering. Vol. 4(2) p. 135–143. DOI 10.1061/(ASCE)1084-0699(1999)4:2(135).
- Halton J.H. 1970. Retrospective and prospective survey of the Monte Carlo method. SIAM Review. Vol. 12(1) p. 1–63.
- Heidrich Z., Witkowski A. 2010. Urządzenia do oczyszczania ścieków: Projektowanie, przykłady obliczeń [Equipment for wastewater treatment: Design, examples of calculations]. Warszawa. Wydaw. Seidel-Przywecki. ISBN 978-83-60956-14-4 pp. 320.
- HENDREN C.O., BADIREDDY A.R., CASMAN E., WIESNER M.R. 2013. Modeling nanomaterial fate in wastewater treatment: Monte Carlo simulation of silver nanoparticles (nano-Ag). Science of Total Environment. Vol. 449 p. 418–425. DOI 10.1016/j. scitotenv.2013.01.078.
- JAGIEŁŁO R., BEKER C., JAGODZIŃSKI A.M. 2016. Ocena zgodności rozkładów empirycznych pierśnic drzewostanów bukowych różnych klas wieku z wybranymi rozkładami teoretycznymi [Goodness of fit evaluation of the breast height diameter distributions of beech stands differing in age with selected theoretical distributions]. Sylwan. Vol. 160(2) p. 107–119.
- JANOSZ-RAJCZYK M. 2008. Badania wybranych procesów oczyszczania ścieków [Research on selected wastewater treatment processes]. Częstochowa. Wydawnictwo Politechniki Częstochowskiej. ISBN 9788371933837 pp. 249.
- JÄNTSCHI L., BOLBOACĂ S.D. 2018. Computation of probability associated with Anderson–Darling statistic. Mathematics. Vol. 6 p. 88–104. DOI 10.3390/math6060088.
- KACZOR G. 2009. Concentrations of the pollutants in the sewage drained from the rural sewerage systems in lesser Poland voivodship. Infrastructure and Ecology of Rural Areas. Vol. 9 p. 97–104.
- KACZOR G., BERGEL T., BUGAJSKI P., PIJANOWSKI J. 2015. Aspects of sewage disposal from tourist facilities in national parks and other protected areas. Polish Journal of Environmental Studies. Vol. 24 p. 107–114. DOI 10.15244/pjoes/28355.
- KLIMIUK E., ŁEBKOWSKA M. 2003. Biotechnologia w ochronie środowiska [Biotechnology in environmental protection]. Warszawa. Wydaw. Nauk. PWN. ISBN 8301140674 pp. 268.
- Kurek K., Bugajski P., Operacz A., Młyński D., Wałęga A. 2020.

 Technological reliability of sewage treatment plant with the Pomiltek Mann type bioreactor. Journal of Water and Land Development. Vol. 46 p. 146–152. DOI 10.24425/jwld.2020. 134207.
- Łomotowski J., Szpindor A. 1999. Nowoczesne systemy oczyszczania ścieków [Modern wastewater treatment systems]. Warszawa. Arkady. ISBN 83-213-4139-x pp. 456.
- MARZEC M. 2017. Reliability of removal of selected pollutants in different technological solutions of household wastewater treatment plants. Journal of Water and Land Development. Vol. 35 p. 141–148. DOI 10.1515/jwld-2017-0078.
- Miksch K., Sikora J. 2021. Biotechnologia ścieków [Wastewater biotechnology]. Warszawa. Wydaw. Nauk. PWN. ISBN 9788301161835 pp. 238.
- MŁYŃSKA A., CHMIELOWSKI K., MŁYŃSKI D. 2017. Analiza zmian jakości ścieków w trakcie procesów oczyszczania na oczyszczalni w Przemyślu [The analysis of the changes in the sewage quality during treatment processes on the wastewater treatment plant in Przemyśl]. Inżynieria Ekologiczna. Vol. 18 p. 18–26. DOI 10. 12912/23920629/74973.
- MEYŃSKI D., BUGAJSKI P., MEYŃSKA A. 2019. Application of the mathematical simulation methods for the assessment of the wastewater treatment plant operation work reliability. Water. Vol. 11(5) p. 873–890. DOI 10.3390/w11050873.
- MŁYŃSKI D., CHMIELOWSKI K., MŁYŃSKA A. 2016a. Ocena skuteczności oraz stabilności pracy oczyszczalni ścieków w Zabajce [The

Assessment of the reliability of the operation of a sewage treatment plant using Monte Carlo simulation

- assesment of the efficency and stability of work sewage treatment plant in Zabajka]. Inżynieria Ekologiczna. Vol. 47 p. 123–130. DOI 10.12912/23920629/62856.
- MŁYŃSKI D., CHMIELOWSKI K., MŁYŃSKA A., MIERNIK W. 2016b. Ocena skuteczności pracy oczyszczalni ścieków w Jaśle [Evaluation of efficency of sewage treatment plant in Jasło]. Infrastruktura i Ekologia Terenów Wiejskich. Vol. 1 p. 147–162. DOI 10.14597/infraeco.2016.1.1.011.
- MŁYŃSKI D., KUREK K., BUGAJSKI P. 2018. An analysis of seasonal waste draining for the urban agglomeration using statistical methods. Water. Vol. 10(8) p. 976–990. DOI 10.3390/w10080976.
- Młyński D., Młyńska A., Chmielowski K., Pawełek J. 2020. Investigation of the wastewater treatment plant processes efficiency using statistical tools. Sustainability. Vol. 12(24) p. 10522–10538. DOI 10.3390/su122410522.
- Moriasi D.N., Arnold J.G., Van Liew M.W., Bingner R.L., Harmel R.D., Veith T.L. 2007. Model evaluation guidelines for systematic quantification of accuracy in watershed simulations. Transactions of the ASABE. Vol. 50(3) p. 885–900. DOI 10.13031/2013.23153.
- Nowobilska-Majewska E., Bugajski P.M. 2019. The analysis of the amount of pollutants in wastewater after mechanical treatment in the aspect of their susceptibility to biodegradation in the treatment plant in Nowy Targ. Journal of Ecological Engineering. Vol. 20(8) p. 135–143. DOI 10.12911/22998993/110393.
- OLIVEIRA S.C., VON SPERLING M. 2007. Reliability analysis of stabilisation pond systems. Water Science & Technology. Vol. 55(11) p. 127–134. DOI 10.2166/wst.2007.343.
- OLIVEIRA S.C., VON SPERLING M. 2008. Reliability analysis of wastewater treatment plants. Water Research. Vol. 42(4–5) p. 1182–1194. DOI 10.1016/j.watres.2007.09.001.
- OLYAEI M.A., KARAMOUZ M., ASCE F., FARMANI R. 2018. Framework for assessing flood reliability and resilience of wastewater treatment plants. Journal of Environmental Engineering. Vol. 144(9) p. 1–14. DOI 10.1061/(ASCE)EE.1943-7870.0001422.
- Płuciennik-Koropczuk E., Jakubaszek A. 2012. Podatność ścieków na rozkład biochemiczny w procesach mechaniczno-biologicznego oczyszczania [Susceptibility of wastewater for biochemical decomposition in mechanical biological wastewater treatment processes]. Zeszyty Naukowe. Inżynieria Środowiska. Vol. 148 (28) p. 73–83.
- PSARROU E., TSOUKALAS I., MAKROPOULOS E. 2018. A Monte-Carlo-based method for the optimal placement and operation scheduling of sewer mining units in urban wastewater networks. Water. Vol. 10 p. 200–223. DOI 10.3390/w10020200.
- Rozporządzenie Ministra Gospodarki Morskiej i Żeglugi Śródlądowej z dnia 12 lipca 2019 r. w sprawie substancji szczególnie szkodliwych dla środowiska wodnego oraz warunków, jakie należy spełnić przy wprowadzaniu do wód lub do ziemi ścieków, a także przy odprowadzaniu wód opadowych lub roztopowych do wód lub do urządzeń wodnych [Regulation of the Minister of Maritime Economy and Inland Navigation of 12 July 2019 on substances particularly harmful to the aquatic environment and conditions to be met during sewage discharge into the water or into the ground and during rainwater or snowmelt discharge into the water or into the water devices]. Dz.U. 2019 poz. 1311.

- Schauberger G., Piringer M., Bauman-Stanzer K., Knauder W., Petz E. 2013. Use of a Monte Carlo technique to complete a fragment set of H₂S emission rates from a wastewater treatment plant. Journal of Hazardous Materials. Vol. 263 p. 694–701. DOI 10.1016/j. jhazmat.2013.10.034.
- SINGH J., KNAPP H.V., ARNALD J.G., DEMISSIE M. 2004. Hydrologic modeling of the Iroquois River watershed using HSPF and SWAT. Journal of the American Water Resources Association. Vol. 41(2) p. 343–360. DOI 10.1111/j.1752-1688.2005.tb03740.x.
- Taheriyoun M., Moradinejad S. 2015. Reliability analysis of a wastewater treatment plant using fault tree analysis and Monte Carlo simulation. Environmental Monitoring and Assessment. Vol. 187 p. 4186–4199. DOI 10.1007/s10661-014-4186-7.
- Todeschini S. 2016. Hydrologic and environmental impacts of imperviousness in an industrial catchment of Northern Italy. Journal of Hydrologic Engineering. Vol. 21(7). DOI 10.1061/(ASCE)HE.1943-5584.0001348.
- Van Liew M.W., Arnold J.G., Garbrecht J.D. 2003. Hydrologic simulation on agricultural watersheds: Choosing between two models. Transactions of the ASAE. Vol. 46(6) p. 1539–15512. DOI 10.13031/2013.15643.
- Wałęga A., Chmielowski K., Młyński D. 2018. Influence of the hybrid sewage treatment plant's exploitation on its operation effectiveness in rural areas. Sustainability. Vol. 10 p. 2689–2706. DOI 10.3390/su10082689.
- Walega A., Chmielowski K., Młyński D. 2019. Nitrogen and phosphorus removal from sewage in biofilter Activated sludge combined systems. Polish Journal of Environmental Studies. Vol. 28 p. 1939–1947. DOI 10.15244/pjoes/89898.
- WALĘGA A., RUTKOWSKA A., POLICHT-LATAWIEC A. 2014. Sensitivity of beta and Weibull synthetic unit hydrographs to input parameter changes. Polish Journal of Environmental Studies. Vol. 1 p. 221–229
- WANG X. (ed.) 2010. Vehicle noise and vibration refinement. Sawstone.Woodhead Publishing. ISBN 978-1-84569-804-1 pp. 416.
- WASIK E., CHMIELOWSKI K. 2013. Evaluation of the operation of the sewage treatment plant, Kujawy in Cracow. Teka Komisji Ochrony i Kształtowania Środowiska Przyrodniczego. Vol. 10 p. 481–488.
- YU G., SAPIRO G., MALLAT S. 2012. Solving inverse problems with piecewise linear estimators: From Gaussian mixture models to structured sparsity. IEEE Transactions on Image Processing. Vol. 21 p. 2481–2499. DOI 10.48550/arXiv.1006.3056.
- Yuan Y., Nie J., McCutcheon S.C., Taguas E.V. 2014. Initial abstraction and curve numbers for semiarid watersheds in south eastern Arizona. Hydrological Processes. Vol. 28 p. 774–783. DOI 10.1002/hyp.9592.
- ZENG X., WANG D., Wu J. 2015. Evaluating the three methods of goodness of fit test for frequency analysis. Journal of Risk Analysis and Crisis Response. Vol. 5(3) pp. 178–187. DOI 10.2991/jrarc.2015.5.3.5.
- ZORAN D., WEISS Y. 2011. From learning models of natural image patches to whole image restoration. International Conference on Computer Vision p. 479–486. DOI 10.1109/ICCV.2011.6126278.